FOR INTERNAL CIRCULATION ONLY

LECTURE NOTES

ON

CONCRETE TECHNOLOGY

DIPLOMA 6th SEMESTER

COMPILED BY

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MODULE I CEMENT

Cement is a binder, a substance that sets and hardens and can bind other materials together. Cements used in construction can be characterized as being either hydraulic or non-hydraulic, depending upon the ability of the cement to be used in the presence of water. Non-hydraulic cement will not set in wet conditions or underwater, rather it sets as it dries and reacts with carbon dioxide in the air. It can be attacked by some aggressive chemicals after setting. Hydraulic cement is made by replacing some of the cement in a mix with activated aluminum silicates, pozzolana, such as fly ash. The chemical reaction results in hydrates that are not very water-soluble and so are quite durable in water and safe from chemical attack. This allows setting in wet condition or underwater and further protects the hardened material from chemical attack (e.g., Portland cement).

Uses:-

- Cement mortar for Masonry work, plaster and pointing etc.
- Concrete for laying floors, roofs and constructing lintels, beams, weather- shed, stairs, pillars etc.
- Construction of water, wells, tennis courts, septic tanks, lamp posts, telephone cabins etc. Making joint for joints, pipes, etc.
- Manufacturing of precast pipes, garden seats, flower posts, etc. Preparation of foundation, water tight floors, footpaths, etc.

ORDINARY PORTLAND CEMENT:-

Ordinary Portland cement is the most common type of cement in general use around the world. This cement is made by heating limestone(calcium carbonate) with small quantities of other materials (such as clay) to 1450°C in a kiln, in a process known as calcinations, whereby a molecule of carbondioxideis liberated from the calcium carbonate to form calcium oxide,or quicklime, which is then blended with the other materials that have been included in the mix. The resulting hard substance, called 'clinker', is then ground with a small amount of gypsuminto a powder to make 'Ordinary Portland Cement'(often referred to as OPC). Portland cement is a basic ingredient of concrete,mortarand most non-specialty grout. The most common use for Portland cement is in the production of concrete. Concrete is a composite material consisting of aggregate(graveland sand), cement, and water. As a construction material, concrete can be cast in almost any shape desired, and once hardened, can become a structural (load bearing) element. Portland cement may be grey or white.

- This type of cement use in construction when there is no exposure to sulphates in the soil or ground water.
- Lime saturation Factor is limited between i.e. 0.66 to 1.02.
- Free lime-cause the Cement to be unsound. Percentage of (AL₂O₃/Fe₂O₃) is not less than 0.66.
- Insoluble residue not more than 1.5%.
- Percentage of SO₃ limited by 2.5% when $C_3A < 7\%$ and not more than 3% when $C_3A > 7\%$.
- Loss of ignition -4%(max)
- Percentage of Mg0-5% (max.) Fineness -not less than 2250 cm²/g.

Chemical constituents of cement:-

These are the different constituents which combine to make cement. These are their percentage content in order to give good cement.

Oxide CaO	Per cent	content 60–67
SiO ₂		17–25
Al ₂ O ₃		3.0-8.0
Fe ₂ O ₃		0.5–6.0
MgO		0.1–4.0 0.4–
Alkalies(K ₂ O,Na ₂ O)	1.3	0.4
SO ₃		1.3–3.0

Hydration of Cement:-

The chemical reaction between cement and water in a proportioning mix is called as hydration of cement. It may be in concrete mix or in the making of mortar in the field work.

SETTING OF CEMENT:-

The action of changing mixed cement from a fluid state to a solid state is called setting of cement and time required for it to set is called setting time of cement. Setting time of cement is same as setting time of concrete.

SETTING TIME OF CEMENT:-

1. Initial Setting Time

Initial Setting Time is defined as the period elapsing between the time when water is added to the cement and the time at which the needle of 1 mm square section fails to pierce the test block to a depth of about 5 mm from the bottom of the mould. A period of 30 minutes is the minimum initial setting time, specified by ISI for ordinary and rapid hardening Portland cements and 60 minutes for low heat cement.

2. Final Setting Time

Final Setting Time is defined as the period elapsing between the time when water is added to cement and the time at which the needle of 1 mm square section with 5 mm diameter attachment makes an impression on the test block. 600 minutes is the maximum time specified for the final set for all the above mentioned Portland cement. IS: 269-1976 specifies the strengths in compression on the standard mortar-cube.

STRUCTURE OF HYDRATED CEMENT:-

The desirable engineering characteristics of hardened concrete —strength, dimensional stability, and durability —are influenced not only by the proportion but also by the properties of the hydrated cement paste, which, in turn, depend on the micro-structural features (i.e., the type, amount, and distribution of solids and voids).

Fresh cement paste is a plastic network of particles of cement in water but, once the cement paste has set, its apparent or gross volume remains approximately constant.

At any stage of hydration, the hardened paste consists of hydrates of the various compounds, referred to collectively as gel, crystals of Ca(OH)2, some minor components, un hydrated cement and the residue of water-filled spaces in the fresh paste.

VARIOUS TESTS ON CEMENT:

Basically two types of tests are under taken for assessing the quality of cement. These are either field test or lab tests. The current section describes these tests in details.

Field test:

There are four field tests may be carried out to as certain roughly the quality of cement. There are four types of field tests to access the colour, physical property, and strength of the cement as described below.

Colour:

- The colour of cement should be uniform.
- It should be typical cement colour i.e. grey colour with a light greenish shade. **Physical properties:**
- Cement should feel smooth when touched between fingers.
- If hand is inserted in a bag or heap of cement, it should feel cool. **Presence of lumps:**
- Cement should be free from lumps.
- For a moisture content of about 5 to 8%, this increase of volume may be much as 20 to 40 %, depending upon the grading of sand. **Strength:**
- A thick paste of cement with water is made on a piece of thick glass and it is kept under water for 24 hours. It should set and not crack. **Physical properties:**
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Laboratory tests:

Six laboratory tests are conducted mainly for assessing the quality of cement. These are: fineness, compressive strength, consistency, setting time, soundness and tensile strength.

Fineness:

- This test is carried out to check proper grinding of cement.
- The fineness of cement particles may be determined either by sieve test or permeability apparatus test.
- In sieve test, the cement weighing 100 gm is taken and it is continuously passed for 15 minutes through standard BIS sieve no. 9. The residue is then weighed and this weight should not be more than 10% of original weight.
- In permeability apparatus test, specific area of cement particles is calculated. This test is better than sieve test. The specific surface acts as a measure of the frequency of particles of average size.

Compressive strength:

- This test is carried out to determine the compressive strength of cement.
- The mortar of cement and sand is prepared in ratio 1:3.
- Water is added to mortar in water cement ratio 0.4.
- The mortar is placed in moulds. The test specimens are in the form of cubes and the moulds are of metals. For 70.6 mm and 76 mm cubes, the cement required is 185gm and 235 gm respectively.
- Then the mortar is compacted in vibrating machine for 2 minutes and the moulds are placed in a damp cabin for 24 hours.
- The specimens are removed from the moulds and they are submerged in clean water for curing.
- The cubes are then tested in compression testing machine at the end of 3days and 7 days. Thus compressive strength was found out.

Consistency:

- The purpose of this test is to determine the percentage of water required for preparing cement pastes for other tests.
- Take 300 gm of cement and add 30 percent by weight or 90 gm of water to it.
- Mix water and cement thoroughly.
- Fill the mould of Vicat apparatus and the gauging time should be 3.75 to 4.25 minutes.
- Vicat apparatus consists of a needle is attached a movable rod with an indicator attached to it. There are three attachments: square needle, plunger and needle with annular collar.
- The plunger is attached to the movable rod.
- The plunger is gently lowered on the paste in the mould.
- The settlement of plunger is noted. If the penetration is between 5 mm to 7 mm from the bottom of mould, the water added is correct.
- If not process is repeated with different percentages of water till the desired penetration is obtained.
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Setting time:

- This test is used to detect the deterioration of cement due to storage. The test is performed to find out initial setting time and final setting time.
- Cement mixed with water and cement paste is filled in the Vicatmould.
- Square needle is attached to moving rod of Vicat apparatus.
- The needle is quickly released and it is allowed to penetrate the cement paste. In the beginning the needle penetrates completely. The procedure is repeated at regular intervals till the needle does not penetrate completely.(up to 5mm from bottom)
- Initial setting time =<30min for ordinary Portland cement and 60 min for low heat cement. The cement paste is prepared as above and it is filled in the Vicatmould.
- The needle with annular collar is attached to the moving rod of the Vicat apparatus.
- The needle is gently released. The time at which the needle makes an impression on test block and the collar fails to do so is noted.
- Final setting time is the difference between the time at which water was added to cement and time as recorded in previous step, and it is =<10hours.

Soundness:

- The purpose of this test is to detect the presence of uncombined lime in the cement.
- The cement paste is prepared.
- The mould is placed and it is filled by cement paste.
- It is covered at top by another glass plate. A small weight is placed at top and the whole assembly is submerged in water for 24 hours.
- The distance between the points of indicator is noted. The mould is again placed in water and heat is applied in such a way that boiling point of water is reached in about 30 minutes. The boiling of water is continued for one hour. The mould is removed from water and it is allowed to cool down.

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- The mould is removed from water and it is allowed to cool down.
- The distance between the points of indicator is again measured. The difference between the two readings indicates the expansion of cement and it should not exceed 10 mm.

Tensile strength:

- This test was formerly used to have an indirect indication of compressive strength of cement.
- The mortar of sand and cement is prepared.
- The water is added to the mortar.
- The mortar is placed in briquette moulds. The mould is filled with mortar and then a small heap of mortar is formed at its top. It is beaten down by a standard spatula till water appears on the surface. Same procedure is repeated for the other face of briquette.
- The briquettes are kept in a damp for 24 hours and carefully removed from the moulds.
- The briquettes are tested in a testing machine at the end of 3 and 7 days and average is found out.

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- The briquettes are tested in a testing machine at the end of 3 and 7 days and average is found out.

Grade of cement:-

Grade of cement represents the specific 28 days compressive strength. The following three grades are given along with their compressive strengths

- 33Grade OPC –33MPa
- 43Grade OPC 43MPa
- 53Grade OPC 53MPa

ADMIXTURE

Admixture is a compound which is used in order to increase or decrease the initial and final setting time ofcement.

Admixtureanditstypes

Chemical admixtures are the ingredients in concrete other than Portland cement, water, and aggregatethose are added to the mix immediately before or during mixing. Producers use admixtures primarily toreduce the cost of concrete construction; to modify the properties of hardened concrete; to ensure

thequality of concreted uring mixing, transporting, placing, and curing; and to overcome certain emergencies during concrete operations.

It isofvarioustypes-

Water-reducing admixtures usually reduce the required water content for a concrete mixture by about 5to 10 percent. Consequently, concrete containing a water-reducing admixture needs less water to reacharequired slump thanuntreatedconcrete. Thetreatedconcrete can have allower water-cementratio. This usually indicates that a higher strength concrete can be produced without increasing the amount ofcement. Recent advancements in admixture technology have led to the development of mid-range waterreducers. These admixtures reduce water content by at least 8 percent and tend to be more stable over awider range of temperatures. Mid-range water reducers provide more consistent setting times than standardwater reducers.

Retarding admixtures, which slow the setting rate of concrete, are used to counteract the accelerating effect of hotweather on concrete setting. High temperatures of ten cause an

increasedrateofhardeningwhich makes placing and finishing difficult. Retarders keep concrete workable during placement anddelay the initial set of concrete. Most retarders also function as water reducers and may entrain someairinconcrete.

Accelerating admixtures increase the rate of early strength development; reduce the time required forproper curing and protection, and speed up the start of finishing operations. Accelerating admixtures are especially useful formodifying the properties of concrete incold weather.

Superplasticizers, alsoknownasplasticizersorhigh-rangewaterreducers(HRWR), reducewatercontentby12 to 30percentand can beaddedtoconcretewithalow-to-normalslump and water-

cement ratio to make high-slump flowing concrete. Flowing concrete is a highly fluid but workableconcrete that can be placed with little or no vibration or compaction. The effect of super plasticizers last sonly 30 to 60 minutes, depending on the brand and dosage rate, and is followed by a rapidloss in workability. As a result of the slump loss, super plasticizers are usually added to concrete at the jobsite.

Corrosion-inhibiting admixtures fall into the specialty admixture category and are used to slowcorrosionofreinforcingsteelinconcrete. Corrosioninhibitors can be used as a defensive strategy for concret e structures, such as marine facilities, highway bridges, and parking garages, that will be exposed to high concentrations of chloride. Other specialty admixtures include shrinkage-reducing admixtures and alkali-silica reactivity inhibitors. The shrinkage reducers are used to control drying shrinkage and minimize cracking, while ASR inhibitors control durability problems associated with alkali-silica reactivity.

DOSASES:-

Super plasticizers are commonly known as High Range Water Reducers because it permits low watercement ratio as well as the workability also affected. In very recent decades, super plasticizers createsmilestonein the advancementof chemical admixtures for Portland cementconcrete. The dramaticeffectof super plasticizer (SP) on propertiesoffresh and hardened concrete hasstudied and theproperties of concrete inspected are compressive strength and slump test. To determine the optimumdosage for the admixture, an experimental investigation conducted and the effect of over dosage of theSP admixture experimented, together with one control mixed. The viscosity of grout and hence theworkability of concrete influenced by the dosage of super plasticizer. From dosages of admixture, thedifference between concrete mixes comes, which used at amounts 400 ml/100 kg, 600 ml/100 kg, 800ml/100 kg, 1000 ml/100 kg and 1200 ml/100 kg of cement were prepared. By dosage 1.0% of SP,compressive strength is improved and after 28 days curing it is 57 N/mm², which is higher than that ofcontrolspecimen.

AGGREGATE

Aggregates are defined as inert, granular, and inorganic materials that normally consist of stone orstone -like solids. Aggregates can be used alone (in road bases and various types of fill) or can be used with cementing materials (such as Portland cement or asphalt cement) to form composite materials or concrete.

Classificationofaggregatesbased on:GrainSize

Ifyou separateaggregates by size, there are two overriding categories: • Fine aggregates

Coarseaggregates

The size of fine aggregates is defined as 4.75mm or smaller. That is, aggregates which can be passedthrough a number 4 sieve, with amesh size of 4.75mm. Fine aggregates include things such as sand, siltand clay. Crushed stoneand crushed gravelmightalsofallunder this category.

Typically, fineaggregates are used to improve work ability of a concrete mix.

Coarse aggregates measure above the 4.75mm limit. These are more likely to be natural stone or gravelthat has not been crushed or processed. These aggregates will reduce the amount of water needed for aconcretemix, which may also reduce

workabilitybutimproveitsinnatestrength.Classificationofaggregatesbasedon: Density Therearethree weight-based variationsofaggregates:

- Lightweight
- Standard
- Highdensity

Different density aggregates will have much different applications. Lightweight and ultra lightweightaggregatesaremoreporousthantheirheaviercounterparts,so they can be puttogreatuseing reenroof

construction, for example.

They are also used in mixes for concrete blocks and pavements, as well as insulation and fire proofing.

High density aggregates are used to form heavy weight concrete. They are used for the density aggregates are used to form heavy weight concrete. They are used for the density aggregates are used to form heavy weight concrete. They are used for the density aggregates are used to form heavy weight concrete. They are used for the density aggregates are used to form heavy weight concrete. They are used for the density aggregates are used to form heavy weight concrete. They are used for the density aggregates are used to form heavy weight concrete. They are used for the density aggregates are used to form heavy weight concrete. They are used for the density aggregates are used to form heavy weight concrete. They are used for the density aggregates are used to denote the density aggregates and the density aggregates are used to denote the density aggregates and the density aggregates are used to denote the density aggregates and the density aggregates are used to denote the density aggregates and the density aggregates are used to denote the density aggregates and the density aggregates aggregate aggregated aggregate aggregate aggregate aggregate aggregate aggregate aggregate aggregated aggregate aggregate aggregated aggregate aggregate aggregated ag

when high strength, durable concrete structures are required

building foundations or pipework ballasting, for example. Classification of aggregates based on: Geographical Origin

Another wayto classifyaggregatesisbytheirorigin. You candothiswithtwogroups:

- Natural Aggregates taken from natural sources, such as riverbeds, quarries and mines. Sand, gravel, stone and rockarethemostcommon, and these can be fine or coarse.
- Processed Also called 'artificial aggregates', or 'by-product' aggregates, they are commonly taken from industrial or engineering waste, then treated to form construction aggregates for highquality concrete. Common processed aggregates include industrial slag, as well as burntclay. Processed aggregates are used for both lightweight and high-density concrete mixes. Classification of aggregates based on: Shape

Shapeis one of themost effective waysof differentiating aggregates. The shape of your chosenaggregates will have a significant effect on the workability of your concrete. Aggregates purchased inbatchesfrom a reputable suppliercanbe consistentin shape, if required, butyou can also mixaggregate shapes if youne edto. The different shapes of aggregates are:

- Rounded Natural aggregates smoothed by weathering, erosion and attrition. Rocks, stone, sandand gravel found in riverbeds are your most common rounded aggregates. Rounded aggregates arethe mainfactorbehindworkability.
- Irregular These are also shaped by attrition, but are not fully rounded. These consist of smallstonesandgravel,andofferreducedworkabilitytoroundedaggregates.
- Angular Used for higher strength concrete, angular aggregates come in the form of crushed rockand stone. Workability is low,but this can be offset by filling voids with rounded or smalleraggregates.
- Flaky Defined as aggregates that are thin in comparison to length and width. Increases surfaceareainaconcretemix.
- Elongated Also addsmore surface area toa mix– meaningmore cementpasteis needed. Elongated aggregates are longer than they are thick or wide.
- Flaky and elongated A mix of the previous two and the least efficient form of aggregate with regards towork ability.

Particleshapeand Texture:-

Aggregateparticle shape and surface texture are important for proper compaction, deformationresistance, and workability. However, the ideal shape for HMA and PCC is different because aggregates serve different purposes in each material. In HMA, since aggregates are relied upon toprovide stiffness and strength by interlocking with one another, cubic angular-shaped particles with arough surface texture are best. However, in PCC, where aggregates are used as an inexpensive high-strength material to occupy volume, workability is the major issue regarding particle shape.

Therefore,inPCCroundedparticlesarebetter.Relevantparticleshape/texturecharacteristicsarediscussedbel ow.

Rounded particles create less particle-to-particle interlock than angular particles and thus provide betterworkability andeasier compaction. However, in HMA less interlock is generally a disadvantage asrounded aggregate will continue tocompact, shove and rutafter construction. Thus angular particles are desirable for HMA (despite their poorer workability), while rounded particles are desirable for PCCbecause of their betterworkability.

FlatorElongatedParticles

These particles tend to impede compaction or break during compaction and thus, may decrease strength.

Smooth-SurfacedParticles

Theseparticleshavealowersurface-to-volumeratiothanroughsurfacedparticlesandthusmaybeeasier to coat with binder. However, in HMA asphalt tends to bond more effectively with rough- surfaced particles, and in PCC rough-surfaced particles provide more area to which the cementpastecanbond. Thus, rough-surface particles are desirable for both HMA and PCC.

ENGINEERINGPROPERTIESOFAGGREGATES:-

Aggregatesareusedinconcreteto provideeconomyin thecostofconcrete. Aggregatesactas filleronly. These donot react with cement and water.

Buttherearepropertiesorcharacteristicsofaggregatewhich influencethepropertiesofresultingconcrete mix. These are as follow.

- 1. Composition
- 2. Size &Shape
- 3. SurfaceTexture
- 4. SpecificGravity
- 5. Bulk Density
- 6. Voids
- 7. Porosity&Absorption
- 8. BulkingofSand
- 9. FinenessModulusofAggregate
- 10. Surface IndexofAggregate
- 11. DeleteriousMaterial
- 12. CrushingValueofAggregate13.ImpactValueofAggregate
- 14. Abrasion Value of Aggregate

1. COMPOSITION

Aggregatesconsistingofmaterialsthatcanreactwithalkaliesincementandcauseexcessiveexpansion, cracking and deterioration of concrete mix should never be used. Therefore it is required totestaggregatestoknowwhetherthereispresenceofanysuchconstituentsinaggregateornot.

2. SIZE&SHAPE

The size and shape of the aggregate particles greatly influence the quantity of cement required inconcrete mix and hence ultimately economy of concrete. For the preparation of economical concretemix on should uselargestcoarse aggregates feasible for the structure. IS-456 suggests following recommendation to decide the maximum size of coarse aggregate to be used in P.C.C&R.C.Cmix.

Maximumsizeofaggregateshouldbelessthan

- One-fourthoftheminimum dimension of the concrete member.
- One-fifthoftheminimum dimension of the reinforced concrete member.
- The minimum clear spacing between reinforced bars or 5mm less than the minimum cover between the reinforced bars and form, which ever is smaller for heavily reinforced concrete members such as the ribs of the main bars.

Remember that the size& shape of aggregate particles influence the properties of freshly mixedconcrete moreas compared to those of hardened concrete.

3. SURFACETEXTURE

The development of hard bond strength between aggregate particles and cement paste depends uponthesurface texture, surface roughness and surface porosity of the aggregate particles.

If the surface is rough but porous, maximum bond strength develops. In porous surface aggregates, the bondstrengthincreases due to setting of cement paste in the pores.

4. SPECIFICGRAVITY

The ratio of weight of oven dried aggregates maintained for 24 hours at a temperature of 100 to 110^oC, to the weight of equal volume of water displaced by saturated dry surface aggregate is known asspecificgravity of aggregates.

Specificgravities are primarily of two types.

- Apparentspecificgravity
- Bulkspecific gravity

Specific gravity is a mean to decide the suitability of the aggregate. Low specific gravity generally indicates porous, weak and absorptive materials, whereas high specific gravity indicates materials of goodquality. Specific gravity of majoraggregates falls within the range of 2.6 to 2.9.

Specificgravityvaluesarealsousedwhiledesigningconcretemix.

5.BULKDENSITY

It is defined as the weight of the aggregate required to fill a container of unit volume. It is generally expressed in kg/litre.

Bulk densityofaggregatesdependsupon thefollowing 3 factors.

- Degreeofcompaction
- Gradingofaggregates
- Shapeofaggregateparticles

6. VOIDS

The empty spaces between the aggregate particles are known as voids. The volume of void equals the difference between the gross volume of the aggregate mass and the volume occupied by the particles alone.

7. POROSITY&ABSORPTION

The minute holes formed in rocks during solidification of the molten magma, due to air bubbles, areknownas pores.Rockscontainingpores arecalledporousrocks.

Water absorption may be defined as the difference between the weight of very dry aggregates and theweightofthesaturatedaggregates withsurfacedryconditions.

Dependingupontheamountofmoisturecontentinaggregates, it can exist in any of the 4 conditions.

- Verydryaggregate (havingnomoisture)
- Dryaggregate(containsomemoistureinitspores)
- Saturatedsurfacedryaggregate(porescompletelyfilledwithmoisturebutnomoistureonsurface)
 Moistorwetaggregates(poresarefilledwithmoistureandalsohavingmoistureonsurface)

8. BULKINGOFSAND

It can be defined as in increase in the bulk volume of the quantity of sand (i.e.fine aggregate) in amoist condition over the volume of the same quantity of dry or completely saturated sand. The ratio ofthevolume ofmoistsanddueto the volume ofsandwhendry,iscalled bulkingfactor.

Finesands bulkmorethancoarsesand

When water is added to dry and loose sand, a thin film of water is formed around the sand particles. Interlocking of air in between the sand particles and the film of water tends to push the particles apartdue to surface tension and thus increase the volume. But in case of fully saturated sand the water films are broken and the volume becomes equal to that of drysand.

9. FINENESSMODULUS

Fineness modulus is an empirical factor obtained by adding the cumulative percentages of aggregateretained on each of the standard sieves ranging from 80 mm to 150 micron and dividing this sum by 100.

Fineness modulus is generally used to get an idea of how coarse or fine the aggregate is. More finenessmodulus value indicates that the aggregate is coarser and small value of fineness modulus indicates thatthe aggregateisfiner.

10. SPECIFICSURFACE OFAGGREGATE

The surface area per unit weight of the material is termed as specific surface. This is an indirectmeasure of the aggregate grading. Specific surface increases with the reduction in the size of aggregateparticle. The specific surface area of the fine aggregate is very much more than that of coarse aggregate.

11. DELETERIOUSMATERIALS

Aggregates should not contain any harmful material in such a quantity so as to affect the strength anddurability of the concrete. Such harmful materials are called deleterious materials. Deleterious materials may cause one of the following effects

- Tointerferehydrationofcement
- Topreventdevelopmentofproperbond
- To reducestrengthanddurability
- Tomodifysettingtimes

Deleterious materialsgenerallyfoundinaggregates,maybegroupedasunder

- Organicimpurities Clay, silt&dust
- Saltcontamination

12. CRUSHINGVALUE

The aggregates crushing value gives a relative measure of resistance of an aggregate to crushing undergradually applied compressive load. The aggregate crushing strength value is a useful factor to knowthe behaviour of aggregates when subjected to compressive loads.

13. IMPACTVALUE

The aggregate impact value gives a relative measure of the resistance of an aggregate to sudden shockorimpact. Theimpactvalue of an aggregate is sometime used as an alternative to its crushing value.

14. ABRASIONVALUE OFAGGREGATES

The abrasion value gives a relative measure of resistance of an aggregate to wear when it is rotated in acylinderalongwithsomeabrasive charge.

SIEVEANALYSIS:-

Sieveanalysisisatechniqueusedtodeterminetheparticlesizedistributionofapowder. Thismethod is performed by sifting a powder sample through a stack of wire mesh sieves, separating it into discretesizeranges. Asieveshakerisusedtovibratethesievestackfora specificperiod oftime.

Sieve analysis is important for analyzing materials because particle size distribution can affect a widerange of properties, such as the strength of concrete, the solubility of a mixture, surface area propertiesandeventheirtaste.

FINENESSMODULUS:-

The *Fineness Modulus*(FM) is an empirical figure obtained by adding the total percentage of thesample of an aggregate retained one achofaspecified series of sieves, and dividing the sumby 100.

Fineness modulus of sand (fine aggregate) is an index number which represents the mean size of theparticles in sand. It is calculated by performing sieve analysis with standard sieves. The cumulative percentage retained on each sieve is added and subtracted by 100 gives the value of fineness modulus.

Fineness modulus is generally used to get an idea of how coarse or fine the aggregate is. More finenessmodulus value indicates that the aggregate is coarser and small value of fineness modulus indicates thatthe aggregateisfiner.

THEGRADINGCURVE:-

The grading curve graphically represents the proportion of different grain sizes which the aggregate iscomposed of and which form part of the shortcrete mix. It provides useful information to find out: Whetherthedistribution of the different aggregates is suitable for pumping.

GRADINGOFAGGREGATES:-

Grading of aggregates is determining the average grain size of the aggregates before they are used inconstruction. This is applied toboth coarse and fine aggregates. The aggregate sample is sievedthroughasetofsieves and weights retained one ach sieve in percentage terms are summed up.

GAPGRADINGAGGREGATE:-

Gap grading isdefined as a grading inwhich one ormore intermediate size fractions are absent. Onagradingcurve, itrepresents a horizontal lineoverthe range of sizes that are absent.

MAXIMUM AGGREGATE SIZE:-

Typically, coarse aggregate sizes are larger than 4.75 mm (5 mm in British code), while fine aggregates form the portion below 4.75 mm. A maximum size up to 40 mm is used for coarse aggregate in most structural applications, while for mass concreting purposes such as dams, sizes up to 150 mm may be used.

MODULE II FRESHCONCRETE Workability is the property of concrete which determines the amount of internal work necessary toproduce full compaction. It is a measure with which concrete can be handled from the mixer stage toits finalfullycompacted stage.

FACTORSAFFECTINGWORKABILITYOFCONCRETE:-

01. WaterContentoftheConcreteMix:

Water content will haveimportant influences on the workability in given volume of concrete. The higher thewater content percubic meter of concrete, the higher will be the fluidity of concrete, which affect the workability.

Waterrequirementismainly associated with absorption by aggregates surface & filling up the voids between aggregates.

- Thestrengthofthe concretemaygetreduced.
- Morequantity of water comes out from the surface of concrete resulting into bleeding.
- $\bullet \quad Cements lurry also escapes through the joints of formwork resulting into the loss of cement from concrete$

02. The Size of Aggregates:

Workabilityismainlygovernedbythemaximumsizeofaggregates. Water and pasterequire, willbenot less if a chosen size of aggregates for concrete is bigger. Consequently, for a given quantity of watercontent & paste, biggersize aggregate will give higher workability.

Note: Onthesite, themaximumsizeof aggregateto beusedwilldepend upon themanyfactorssuch asthehandling, mixing and placing equipment, the thickness of section and quantity of reinforcement. Latertwo are very important.

03. The shape of Aggregates:

Angular, flaky& elongated aggregate reduces the work ability of concrete.

Roundedorsub-rounded aggregatesincreasetheworkabilitydueto thereductionofsurfaceareaforagivenvolumeorweight. Therefore, an excess pasteis available to give better lubricating effect.

Roundedshapeaggregatehas

lessfrictionalresistanceandgivesahighworkabilityascomparedtoangular,flakyorelongatedaggregates.

Note: Riversand &gravelprovidegreaterworkabilitytoconcretethancrushedsand.

04. SurfaceTextureofAggregates:

Theroughlytexturedaggregateshavemore surfaceareathansmoothlyroundedaggregatesofthesamevolume. Smooth rounded or glassy aggregates will give better workability than roughly texturedaggregates. A reduction of inter particle frictional resistance offered by smooth aggregates also contributes to higher workability.

05. The Porosity of Aggregates:

Porous and non-saturated aggregate will require more water than non-absorbent aggregates. For thesame degree of workability, latter will require less water. Overall, this factor is only of secondaryimportance.

06. GradingofAggregates:

Grading of aggregates has the greatest influence on workability. The better the grading of aggregates, the less is the amount of void in concrete so well-graded aggregates should be used. When total voids are less inconcrete, the excess paste is available to give better lubricating effect.

With excess amount of concrete paste present in the mixture, it becomes cohesive and fatty that prevents segregation of particles & least amount of compacting efforts is required to compact the concrete.

Foragivenworkability,

there is one value of coarse aggregate/Fine aggregate ratio, which needs the lower water content.

07. UsesofConcreteAdmixtures:

This is one of the commonly used methods to enhance the workability of concrete. Concreteadmixturessuchasplasticizer and superplasticizersgreatlyimprovetheworkability.

Air entraining agents are also used to increase the workability. Air entraining agents creates a largenumber of very tinyair bubbles. These bubbles get distributed throughout the mass of concrete and actas rollers and increase the workability.

Mineral admixtures like Pozzolanic materials are also used to improve the work ability of concrete.

08. AmbientTemperature:

Inhotweather, iftemperatureincreases, theevaporation rate of mixing water also increases and hence fluid viscosity increases, too. This phenomenon affects the flowability of concrete and due to fasthydration of concrete; it will gain strengthear lier which decreases the work ability of fresh concrete.

MeasurementofWorkability:-

The followings tests for workability of concrete gives a measure of workability, which is applicable specifically concerning some particular methods. They bear no relationship to any of the commonmethods of placing and compacting concrete. So, the test results are only relative and should not begiven any absolute measurement. We need tounderstand that each test has their importance, and assuch there is no unique test to measure the workability of concrete in total. The significant advantage is the simplicity of the procedure with an ability to detect variation in the uniformity of a mix of givennominal proportion.

Typesoftestsforworkability:-

- SlumpTest
- CompactingFactorTest FlowTest
- Vee-BeeConsistometerTest

SLUMPTEST:-

The slump test is a means of assessing the consistency of fresh concrete. It is used, indirectly, as ameans of checking that the correct amount of water has been added to the mix.

The steel slump cone is placed on a solid, impermeable, level base and filled with the fresh concrete inthree equal layers. Each layer is rodded 25 times to ensurecompaction. The third layer is finished offlevel with the top of the cone. The cone is carefully lifted up, leaving a heap of concrete that settles or 'slumps' slightly. The upturned slump cone is placed on the base to act as a reference, and the difference in level between its top and the top of the concrete is measured and recorded to the nearest 10 mm to give the slump of the concrete.

When the cone is removed, the slump may take one of three forms. In a true slump the concrete simplysubsides, keeping more or less to shape. In a shear slump the top portion of the concrete shears off andslips sideways. In a collapse slump the concrete collapses completely. Only atrue slump is of any usein the test. If a shear or collapse slump is achieved, afresh sampleshould be taken and the test repeated. A collapse slump will generally mean that the mix is too wet or that it is a high workability mix, for which has the flow test (see separate entry) is more appropriate.



CompactingFactorTest:-

The compacting factor test is designed primarily for use in the laboratory but it can also be used in thefield. It is more precise and sensitive than the slump test and is particularly useful for concrete mixes ofvery low workability as are normally used when concrete is to be compacted by vibration. Such dryconcrete are insensitive to slumptest.

The degree of compaction, called the compacting factor is measured by the density ratio i.e., the ratio of the density actually achieved in the test to density of same concrete fully compacted. The sample ofconcrete to be tested is placed in the upp r hopper up to the brim. The trap-door is opened so that the concrete falls into the low r hopper. Then the trap-door of the lower hopper is opened and the concrete allowed to fall into the cylinder. In the case of a dry-mix, it is likely that the concrete may not fall onopening the trap-door. In such a case, a slight poking by a rod may be required to set the concrete inmotion. The excess concrete remaining above the top level of the cylinder is then cut off with the helpof plane blades supplied with the apparatus. The outside of the cylinder is wiped clean. The concrete is filled up exactly up to the top level of the cylinder. It is weighed to the nearest 10 grams. This weight isknownas—Weightofpartiallycompactedconcrete.

The cylinder is emptied and then refilled with the concrete from the same sample in layers approximately 5 cm deep. The layers are heavily rammed or preferably vibrated so as to obtain full compaction. The top surface

of the fully compacted concrete is then carefully struck off level with thetop of the cylinder and weighed to thenearest 10 gm. This weightis known as Weight of fullycompactedconcrete.

The compaction factor = Weight of partially compacted concrete Weight of fully compacted concrete

FLOWTEST

Thisisalaboratorytest, which gives an indication of the quality of concrete with respect to consistency, cohesiveness a d the proneness to segregation. In this test, a standard mass of concrete is subjected to joint. The spread or the flow of the concrete is measured and this flow is related toworkability.

It can be seen that the apparatus consists of flow table, about 76 cm. in diameter over which concentriccircles are marked. A mould made from smooth metal casting in the form of a frustum of a cone is usedwith the following internal dimensions. The baseis 25 cm. in diameter, upper surface 17 cm. indiameter, and height of the cone is 12 cm. The table top is cleaned of all gritty material and is wetted. The mould is kept on the centre of the table, firmly held and is filled in two layers. Each layer is rodded25 times with a tampingrod 1.6 cm in diameter and 61 cm long rounded at thelower tamping end. After the top layer is rodded evenly, the excess of concrete which has over flowed the mouldisremoved. The mould is lifted vertically upward and the concrete stands on its own without support. Thetable is then raised and dropped 12.5 mm 15 times in about 15 seconds. The diameter of the spreadconcrete is measured in about 6 directions to the nearest 5 mm and the average spread is noted. Theflow of concrete is the percentage increase in the average diameter of the spread concrete over the basediameter of themould.

• Flowper cent=Spreaddiameterincm – 25X100

25

The value could range anything from 0 to 150 per cent. A close look at the pattern of spread of concretecanalso give agoodindication of the characteristics of concrete such as tendency for segregation.

VEE-BEE CONSISTOMETERTEST

This is a good laboratory test to measure indirectly the workability of concrete. This test metal cylindrical pot of the con istomeconsists of a vibrating table, a metal pot, a sheet metal cone, a standardiron rod. Slump test as described earlier is performed, placing the slump cone inside the sheetter. The glass disc attached to the swivel armis turned and placed on the top of the concrete in the pot.

The electrical vibratoristhens witched on and simultaneously a stop watch started.

The vibration is continued till such a time as the conical shape of the concrete disappears and theconcrete assumes a cylindrical shape. This can be judged by observing the glass disc from the top fordisappearance of transparency. Immediately when the concrete fully assumes a cylindrical shape, thestop watch is switched off. The time required for the shape of concrete tochange from slump coneshape to cylindrical shape in seconds is known as Vee Bee Degree. This method is very suitable forvery dry concrete whose slump value cannot be measured by Slump Test but the vibration is toovigorousforconcrete witha slumpgreaterthanabout50mm.

SETTINGTIMEOFCEMENT:-

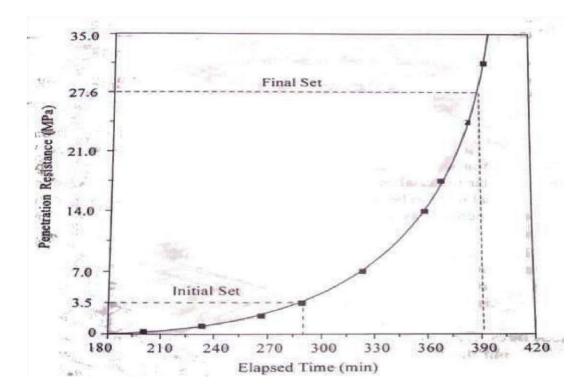
The concrete setting time mostly depends upon the w/c ratio, temperature conditions, type of cement, use of mineral admixture, use of plasticizer, in particular, retarding plasticizer. The significance of setting parameter of concrete is more important for site engineers than setting time of cement. Forkeeping the

concrete we use retarding plasticizers, which increases setting time and the duration up towhichconcreteremainsinthe plasticconditionis ofspecialinterest.

The concrete setting time is determined by using a penetrometer test.

The testprocedure involves,

- 1. Takingasufficientquantityoffreshconcretemixsampleandsievesitthrough 4.75 mmsieve.
- 2. Themortarsamplepassedthroughthesieveiscollected.
- 3. This mortaristhen compacted by rodding, tapping, rocking or by vibrating.
- 4. Levelthesurfaceandkeepitcoveredtopreventthelossofmoisture.
- 5. Removebleedingwater, if any, using a pipette. Insertaneed le of appropriate size, depending upon the degree of the setting of the mortarin the following manner.
- 6. Bringthebearingsurfaceoftheneedleincontactwiththemortarsurface.
- 7. Graduallyanduniformlyapplyaverticalforcedownwardsontheapparatus untiltheneedlepenetratestoadepthof25± 1.5on,as indicatedbythescribemark.
- 8. Thetimeistakentopenetrate25mmdepthcould beabout10seconds.
- 9. Recordtheforcerequiredtoproduce25mmpenetrationandthetimeofinsertingfromthetowaterisaddedtothec ement.
- 10. Calculate the penetration resistance by dividing the recorded force by the bearing area of the needle. This is the penetration resistance.



- Plotagraphofpenetrationresistanceasordinateandelapsedtimeasabscissa.
- Testconductedmustdetermine6penetrationresistances.
- Continuethetestsuntilonepenetrationresistanceofatleast27.6MPaisreached.
- Plotthesepenetrationresistancevaluesonthegraphand connecteach point.
- Nowdrawahorizontallinefrompenetrationresistanceequalto3.5MPa.
- Thepoint ofintersection of this with the smooth curve is read on the x-axis which gives the initial setting time.

• Similarly, ahorizontallineisdrawnfromthepenetrationresistanceof27.6MPaandpointitcutsthesmoothcurveisreadonth e x-axis whichgives thefinalset.

Effectoftimeandtempinworkability:-

Temperature decreases the setting time by increasing hydration rate and that increase the early agestrengthoftheconcrete. This is an advantage that less time will be required before removing of formworks on site, but this decrease the use of proper placement of concrete in the initial stages.

It indicates that the temperature has a negative effect on the workability of concrete as well as strengthup to some extent. Temperature decreases the setting time by increasing hydration rate andthatincrease the early agest rength of the concrete.

This is an advantage that less time will be required before removing of form works on site but

This is an advantage that less time will be required before removing of form works on site, but this decrease the use of proper placement of concrete in the initial stages. And if concrete is not properly laid, then strength distribution will not remain the same throughout the cross-section.

SEGREGATION:-

Thetendencyofseparationofcoarseaggregatesgrainsfromteconcretemassiscalled segregation.

BLEEDING:-

Thetendencyofwaterto risetothesurface offreshlylaid concreteisknownasbleeding.

MIXING ANDVIBRATIONOFCONCRETE:

Mixing is the uniform incorporation of the ingredients within the concrete mix and vibration usuallymeans the mechanical process to assistin the removal of any entrapped air. The air entrapment causes ahoneycombeffectwhichweakens the concrete,

There are calculations and processes for concrete to allow for movement, which often translates tovibrationduetofrictionorthedissimilarityofmaterials, aserious concernasit would be the cause for structural fatigue and failure.

The stagesofconcrete productionare:

- 1. Batchingormeasurementofmaterials
- 2. Mixing
- 3. Transporting
- 4. Placing
- 5. Compacting
- 6. Curing
- 7. Finishing

BatchingofMaterials

For good quality concrete a proper and accurate quantity of all the ingredients should be used. The aggregates, cement and water should be measured with an accuracy of 3 per cent of batch quantity and the admixtures by 5 per cent of the batch quantity. There are two prevalent methods of batchingmaterials, the volume batching and the weigh batching. The factors affecting the choice of batchingmethodare the

size of job, required production rate, and required standards of batching performance. Formostimportantworks weighbatching is recommended.

- a) VolumeBatching
- b) Weigh

BatchingMixing

- 1. HandMixing
- 2. MachineMixing
- a) TiltingMixers
- b) Non-tiltingMixer
- c) ReversingDrumMixer
- d) Pan-typeorStirringMixer
- e) TransitMixer

ChargingtheMixerandMixingTime

Theorderoffeedingtheingredientsintothemixerisas follows:

About 25 per cent of water required for mixing is first introduced into the mixer drum to prevent anysticking of cement on the blades and bottom of the drum. Then the ingredients are discharged throughthe skip. In the skip the sequence ofloading shouldbe to add first half the coarse aggregate then halfthe fine aggregate and over this total cement and then the balance aggregates. After discharging theingredients into the drum the balance water is introduced. The mixing time is counted from the instantcomplete waterisfedintothemixer.

The speed of the mixers is generally 15 to 20 rpm. For proper mixing, the number of revolutions perminute requiredbythe drumare25to30. Time of mixing also depends on capacity of mixer.

A poor quality of concrete is obtained if the mixing time is reduced. On the other hand if the mixing time is increased it is uneconomical. However, it is found that if the mixing time is increased to 2minutesthecompressivestrengthofconcreteproducedisenhancedandbeyondthistimetheimprovement in compressive strength is insignificant. A prolonged mixing may cause segregation. Also, due to longer mixing periods the water may get absorbed by the aggregates or evaporate resultinginloss ofworkabilityandstrength. Transporting

Concreteshouldbetransported to the place of deposition at the earliest without the loss of homogeneity obtained at the time of mixing. A maximum of 2 hours from thetime of mixingispermittedif truckswithagitatorand1hourif truckswithoutagitators are usedfortransportingconcrete. Also it should be ensured that segregation does not take place during transportation and placement. The methods adopted for transporting concrete depend upon the size and importance of thejob, the distance of the deposition mixing the place, and the nature of the terrain. Some ofthe methodsoftransportingconcreteareasbelow:

- a. MortarPan
- b. WheelBarrow
- c. Chutes
- d. Dumper
- e. BucketandRopeway

- f. Beltconveyor
- g. Skipand Hoist
- h. Pumping

Placing:

To achieve quality concrete it should be placed with utmost care securing the homogeneityachieved during mixing and the avoidance of segregation in transporting. Research has shown that adelayed placing of concrete results in a gain in ultimate compressive strength provided the concrete canbe adequately compacted. For drymixesin hot weather delay of half to one hour isallowed whereasforwetmixesincoldweatheritmaybeseveralhours.

Compaction

After concrete is placed at the desired location, the next step in the process of concrete production is its compaction. Compaction consolidates fresh concrete within the moulds or frameworks and aroundembedded parts and reinforcement steel. Considerable quantity of air is entrapped in concrete during its production and there is possible partial segregation also. Both of these adversely affect the quality of concrete. Compaction of the concrete is the process to getrid of the entrappedair and voids, elimination of segregation occurred and to form a homogeneous densemass. It has been found that 5 per centvoids in hardened concrete reduce the strength by over 30 per cent and 10 per centvoids reduce the strength by over 50 percent. Therefore, the density and consequently the strength and

durability of concrete largely depend upon the degree of compaction. For maximum strength driestpossible concretes hould be compacted 100 percent.

The compaction of concrete can be achieved by the following methods.

- 1. HandCompaction
- 2. Compaction by Vibration
 - a. NeedleVibrator:
 - b. Formwork Vibrator
- 3. CompactionbySpinning
- 4. CompactionbyJolting
- 5. Compaction by Rolling Curing

Cement gains strength and hardness because of the chemical action between cement and water. This chemical reaction requires moisture, favourable temperature and time referred to as the curingperiod. Curing of freshly placed concrete is very important for optimum strength and durability. Themajor part of the strength in the initial period is contributed by the clinker compound C₃S and partly byC₂S, and is completed in about three weeks. The later strength contributed by C₂S is gradual and takeslong time. As such sufficient water should be made available to concrete to allowit to gain fullstrength. *The process of keeping concrete damp for this purpose is known as curing.* The object is toprevent the loss of moisture from concrete due to evaporation or any other reason, supply additionalmoisture or heat and moisture to accelerate the gain of strength. Curing must be done for at least threeweeksandinnocaseforlessthantendays.

Approximately 14 litres of water is required to hydrate each bag of cement. Soon after the concrete isplaced, the increase in strength is very rapid (3 to 7 days) and continues slowly thereafter for an indefinite period. Concrete moist cured for 7 days is about 50 per cent stronger than that which is exposed todry air for the entire period. If the concrete keptdamp for one month, the strengthisaboutdoublethanthatofconcrete exposed only to dryair.

MethodsofCuring:

Concrete may be kept moist by a number of ways. The methods consist in either supplying additional moisture to concrete during early hardening period by ponding, spraying, sprinkling, etc. orby preventing loss of moisture from concrete by sealing the surface of concrete by membrane formed by curing compounds. Following are some of the prevalent methods of curing.

- 1. WaterCuring
- 2. SteamCuring
- 3. CuringbyInfraRedRadiation:
- 4. ElectricalCuring
- 5. ChemicalCuring:

Finishing:

Concrete is basically used because of its high compressive strength. However, the finish of theultimate product is not that pleasant. In past couple of decades efforts have been made to developsurfacefinishes togive a betterappearance toconcretesurfacesandare as follows.

- 1. FormworkFinishes
- 2. SurfaceTreatments
- 3. AppliedFinishes

QUALITYOFMIXINGWATER:-

The common specifications regarding quality of mixing water is water should befitfor drinking. Such water should have inorganic solid less than 1000 ppm. This content lead to a solid quantity 0.05% of mass of cement when w/cratio is provided 0.5 resulting small effect on strength.

HARDENEDCONCRETE

The water–cementratio is the ratio of the weight of water to the weight of cementused inaconcretemix. A lower ratio leads to higher strength and durability, but may make the mix difficult toworkwithandform. Workability can be resolved with the use of plasticizers or super-plasticizers.

A maximum of 0.5 ratio when concrete is exposed to freezing and thawing in a moist condition or todeicing chemicals, and a maximum of 0.45 ratio for concrete in a severe or very severe sulphatecondition.

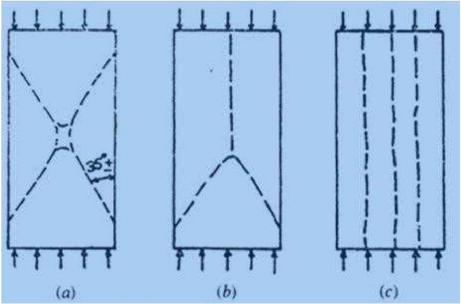
However, a mix with a ratio of 0.35 may not mix thoroughly, and may not flow well enough to beplaced. More water is therefore used than is technically necessary to react with cement. Water—cementratios of 0.45 to 0.60 are more typically used. For higher-strength concrete, lower ratios are used, alongwithaplasticizertoincreaseflow ability.

Natureofstrengthofconcrete:-

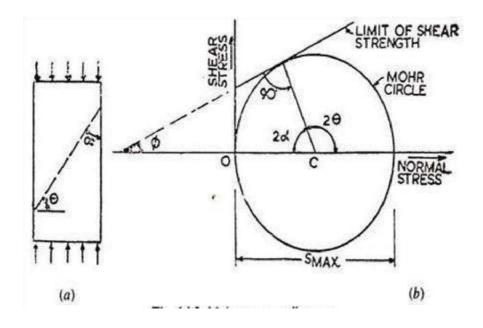
The strength can be defined as the ability to resist force. With-regard to concrete for structural purposesit can be defined as the unit force required to cause rupture. Strength is a good index of most of theother properties of practical importance. In general stronger concretes are stiffer, more water tight andmore resistanttoweatheringetc.

Rupture of concrete may be caused by applied tensile stress, shearing stress or by compressive stress ora combination of two of the above stresses. Concrete being a brittle material is much weaker in tensionand shear than compression and failures of concrete specimens under compressive load are essentially shearfailures on oblique planes as shown in fig.

It is called as shear or cone failure. As the resistance to failure is due to both cohesion and



internal friction, the angle of rupture is not 45° (plane of maximum shear stress), but is a function of the angle of internal friction. It can be shown mathematically that the angle ϕ which the plane of failure makes with the axis of loading is equal to $(45^{\circ} - \phi/2)$ as shown in fig.



The angle of internal friction ϕ of concrete beingof the order of 20°, the angle of inclination of the conventional tests per cimenis approximately 35° as shown in fig.

Maturityofconcrete:-

Concrete maturity is an index value that represents the progression of concrete curing. It is based on an equation that takes into account concrete temperature, time, and strength gain. Concrete maturity is an accurate way to determine real-time strength values of curing concrete.

TensionVs.CompressionofConcrete:-

Concrete has enormous compressive strength, the ability to withstand heavy weights or forces on it. Italso gains strength as it ages. Concrete will solidify in a few hours and harden or set in a few days, butcontinues to gain strength for at least 28 days. Some very thick concrete structures, like dams, willcontinue togainstrengthformonths oryears.

Concrete has almost no tensile strength, the ability to withstand pressing or stretching. Put a boardbetween two supports and press down on the centre. It will bend. The top of the board is undercompression, the bottom which bends is under tension. Concrete can resist the compression, but willbreak under the tension. Concrete cracks in roads and slabs are largely due to tension; different weightsindifferentareas producetensileforces.

The tension to compression ratio for concrete is about 10 to 15 percent. That is, it can withstand about 10 times the pushingforce or compression of the pullingforce or tension. Both strengths increasewith age, but the ratio is steady. Portland cement concrete less than a year old has compressionstrength of 1,000 pounds per square inch (psi) and tension strength of 200 psi. Concrete more than ayearoldhas compressionpsiof2,000pounds andtensionpsiof400. Factorsaffectingstrengthofconcrete:

Concrete strength is affected by many factors, such as quality of raw materials, water/cement ratio,coarse/fine aggregate ratio, age of concrete, compaction of concrete, temperature, relative humidity and curing of concrete.

Relation betweentensile and compressive strengthofconcrete:-

The theoretical compressive strength of concrete is eight times larger than its tensile strength.

Thisimpliesafixed relation between the compressive and tensile strength of concrete. In fact there is a closer elation but not a direct proportionality. The ratio of tensile to compressive strength is lower for higher compressive strengths.

Thismaybeduetothefollowingtworeasons:

- (a) Formationofinferior qualitygelduetoimpropercuring.
- (b) Developmentofmoreshrinkagecracksduetoimpropercuring. Theusesofpozzolanic materials have shown their crease intensile strength.

CuringofConcrete:-

Curing of Concrete is a method by which the concrete is protected against loss of moisture required forhydration and kept within the recommended temperature range. Curing will increase the strength anddecrease thepermeabilityofhardenedconcrete.

MOD III

TESTINGOFHARDENEDCONCRETE

The compressive strength of concrete is considered the basic character of the concrete. Consequently, it is known as the characteristic compressive strength of concrete (fck) which is defined as that value below which not more than five percent of test results are expected to fall based on IS:456-2000. In this definition the test results are based on 150 mm cubecured in water under temp. of 27 \pm 2°C for 28 days and tested in the most saturated condition under direct compression.

Other strength like, direct tensile stress, flexural stress, shear stress and bond stress also are directlyproportional to the compressive stress. Higher is the compressive stress, higher is other stresses also. Not only stresses, other properties for example modulus of elasticity, abrasion and impact resistances, durability are also taken to be related to the compressive strength, hence, the compressive strength is an index of overall quality of concrete.

FactorsAffectingCompressiveStrength:-

Among the materials and mix variables, water -cement ratio is the most important parameter governingthecompressivestrength. Besides W/Cratio, following factors also affect the compressive strength.

- I. Thecharacteristics of cement.
- II. Thecharacteristics and properties of aggregates. III. The degree of compaction
- IV. Theefficiencyofcuring
- V. Ageatthetimeoftesting.VI.Co nditionsoftesting.

TENSILESTRENGTH:

- Tensile strength of concrete underdirecttensionis verysmall and generally neglectedinnormal design practice. Although the value ranges from 8 to 12% of its compressive strength. An average value 10% is the proper choice. The direct tension method suffers the problem likeholding the specimen properly in the testing machine and the application of uniaxial tensile loadnotbeing free of eccentricity.
- Thetensilestrengthcanbecalculatedindirectlybyloadingaconcretecylindertothecompressiveforce alongthe twoopposite ends(withitsaxishorizontal)
- Due to uniform tensile stress acting horizontally along the length of cylinder, the cylinder splitsinto two halves. The magnitude of this tensile stress (acting in a direction perpendicular to theline ofactionofappliedcompression)isgivenby

The indirecttensilestressisknown as splitting tensile strength.

FLEXURALSTRENGTH

- The maximum tensile stress resisted by the plain concrete in flexure (bending) is called flexuralstrength(ormodulus ofrupture)expressedinN/mm2orkg/m².
- The most common plain concrete subjected to flexure is a highway/runway pavement. Thestrengthofpavementconcreteisevaluatedbymeansofbendingonbeamspecimen.
- The flexural strength (modulus of rupture) is determined by testing standard test specimens of 150 mm x 150 mm x 700 mm over a span of 600 mm or 100 mm x 100 mm x 500 mm over aspanof 400 min.undersymmetrical two point loading.

SHEARSTRENGTH:

- Shear strength is the capacity of concrete to resist the sliding of the section over the adjacent section. A good amount of shear strength capacity is possessed by concrete depending upon the grade of concrete and percentage of tensile reinforcement in the section.
- It is difficult to obtain shear strength of concrete but I.S. code suggests the value for different grade of concrete.

PULLOUTTESTOFCONCRETE:

The pullouttest produces a well defined in the concrete and measure a static strength property of concrete. The equipment is simple to assemble and operate. The compressive strength can be considered as proportional to the ultimate pullout force. The reliability of the test is reported as good.

NON-DESTRUCTIVETESTINGOFCONCRETE(NDTONCONCRETE):-

Non destructive test is a method of testing existing concrete structures to assess the strength anddurability of concrete structure. In the non destructive method of testing, without loading the specimento failure (i.e. without destructing the concrete) we can measure strength of concrete. Now days thismethod has become a part of quality control process. This method of testing also helps us to investigate crackdepth, microcracks and deterioration of concrete.

Non destructive testing of concrete is a very simple method oftesting but it requires skilled and experienced persons having some special knowledge to interpret and analyze test results. DIFFERENT

METHODSOFNON-DESTRUCTIVETESTINGOFCONCRETE:-

Various non-destructive methods of testing concrete have been developed to analyze properties ofhardenedconcrete, which are given below.

1. SURFACEHARDNESSTEST

These are of indentation type, include the Williams testing pistol and impact hammers, and are usedonlyforestimation of concretestrength.

2. REBOUNDHAMMERTEST

The rebound hammer test measures the elastic rebound of concrete and is primarily used for estimation of concrete strengthand for comparative investigation.

3. PENETRATIONANDPULLOUTTECHNIQUES

These include the use of the simbi hammer, spit pins, the Windsor probe, and the pullout test. These measure the penetration and pullout resistance of concrete and are used for strength estimation, but they can also be used for comparative studies.



NDT testonconcrete

4. DYNAMICORVIBRATIONTESTS

These includeres on antifrequency and mechanical sonic and ultrasonic pulse velocity methods. These are used to evaluate durability and uniformity of concrete and to estimate its strength and elastic properties.

5. COMBINEDMETHODS

The combined methods involvingultrasonic pulse velocity and rebound hammer have been used to estimate strength of concrete.

6. RADIOACTIVEANDNUCLEARMETHODS

These include the X-ray and Gamma ray penetration tests for measurement of density and thickness of concrete. Also, the neutron scattering and neutron activation methods are used for moisture and cementcontent determination.

7. MAGNETICANDELECTRICAL METHODS

The magnetic methods are primarily concerned with determining cover of reinforcement in concrete, whereas the electrical methods, including microwave absorption techniques, have been used to measuremoisture content and thickness of concrete.

8. ACOUSTICEMISSIONTECHNIQUES

Thesehavebeenusedto studytheinitiationandgrowthofcracksinconcrete.

ELASTICITY, CREEP, SHRINKAGE

Modulus of Elasticity of Concrete can be defined as the slope of the line drawn from stress of zero to acompressive stress of 0.45f'c. As concrete is a heterogeneous material. The strength of concrete isdependentontherelative proportionand modulus of elasticity of the aggregate.

Dynamic modulus is the ratio of stress to strain under vibratory conditions (calculated from dataobtainedfromeitherfreeorforcedvibrationtests, inshear, compression, orelongation). Itisaproperty of viscoelastic materials.

DynamicModulus:-

The value of modulus of elasticity E_cdetermined by actual loading of concrete is known as static modulus of elasticity. This method of testing is known as destructive method as the specimen isstressed or loaded till its failure. The static modulus of elasticity does not represent the true elastic behaviour of concrete due to the phenomenon of creep. At higher stresses the modulus of elasticity is affected more seriously.

Thus a non-destructive method of testing known as dynamic method is adopted for determining themodulus of elasticity. In this case no stress is applied on the specimen. The modulus of elasticity isdetermined by subjecting the specimento longitudinal vibration at their natural frequency that is why this is known as dynamic modulus.

Poisson'sratio:-

Poisson's ratio isthe ratio oflateralstraintolongitudinalstraininamaterial subjectedtoloading. Poisson's ratio varies between 0.1 for high strength concrete and 0.2 for weak mixes. It isnormallytakenas 0.15forstrengthdesignand0.2forserviceabilitycriteria.

Creep:-

Creep in concrete is defined as the deformation of structure under sustained load. Basically, long termpressure or stress on concrete canmake it change shape. This deformation usually occurs in the direction the force is being applied. Like a concrete column getting more compressed, or a beambending. Creep does not necessarily cause concrete to fail or break apart. When a load is applied to concrete, it experiences an instantaneous elastic strain which develops into creep strain if the load is sustained.

Creepis factoredinwhenconcretestructuresaredesigned.

FactorsAffectingCreep:-

These are thefactorswhich affects creep of concrete.

1. Aggregate2.MixProportions

3. Age of concrete

The magnitude of creep strain is one to three times the value of the instantaneous elastic strain, it is proportional to cement-paste content and, thus, inversely proportional to aggregate volumetric content. The magnitude of creep is dependent upon the magnitude of the applied stress, the age and strength of the concrete, properties of aggregates and cementitious materials, amount of cement paste, size and shape of concrete specimen, volume to surface ratio, amount of steel reinforcement, curing conditions, and environmental conditions.

1. InfluenceofAggregate:

Aggregate undergoes very little creep. It is really the paste which is responsible for the creep. However, the aggregate influences the creep of concrete through a restraining effect on the magnitude of creep. The paste which is creeping under load is restrained by aggregate which do not creep. The stronger theaggregate the more is the restraining effect and hence the less is the magnitude of creep. An increasefrom65to 75% of volumetric content of the aggregate will decrease the creep by 10%.

The modulus of elasticity of aggregate is one of the important factors influencing creep. It can be easilyimagined that the higher the modulus of elasticity the less is the creep. Light weight aggregate showssubstantiallyhighercreepthannormalweightaggregate.

2. InfluenceofMixProportions:

The amount of paste content and its quality is one of the most important factors influencing creep. Apoorer paste structure undergoes higher creep. Therefore, it can be said that creep increases withincrease in water/cement ratio. In other words, it can also be said that creep is inversely proportional tothe strength of concrete. Broadly speaking, all other factors which are affecting the water/cement ratioare also affecting thecreep.

3. InfluenceofAge:

Age at which a concrete member is loaded will have a predominant effect on the magnitude of creep. This can be easily understood from the fact that the quality of gel improves with time. Such gel creepsless, whereas a young gel under load being not so stronger creeps more. Whatis said above is not avery accurate statement because of the fact that the moisture content of the concrete being different atdifferentagealsoinfluencesthemagnitude ofcreep.

Unlike brittle fracture, creep deformation does not occur suddenly upon the application of stress.Instead, strain accumulates as a result oflong-term stress. Therefore, creep is a "timedependent"deformation.It worksontheprincipleofHooke'slaw(stressisdirectlyproportionalto strain).

EffectsofCreeponConcreteandReinforcedConcrete:-

- In reinforced concrete beams, creep increases the deflection with time and may be a criticalconsiderationindesign.
- Ineccentricallyloadedcolumns,creepincreasesthedeflectionandcanloadtobuckling.
- In case of statically indeterminate structures and column and beam junctions creep may relieve the stress concentration induced by shrinkage, temperatures changes or movement of support. Creep property of concrete will be useful in all concrete structures to reduce the internal stressesdue tonon-uniformloadorrestrainedshrinkage.
- In mass concrete structures such as dams, on account of differential temperature conditions attheinteriorandsurface, creepisharmfulandbyitselfmaybeacauseofcrackingintheinterior ofdams. Therefore, all precautions and steps must be taken to see that increase in temperaturedoesnottakeplaceintheinteriorofmassconcretestructure.
- Lossofpre stressdue to creepofconcreteinpre stressedconcretestructure.
- Becauseofrapidconstructiontechniques, concretemembers will experience loads that can be as large as the design loads at very early age; these can cause deflections due to cracking and early age low

elastic modulus. So, creep has a significant effect on both the structural integrityandthe economicimpactthatitwillproduceifpredictedwrong.

Shrinkage:-

The volumetric changes of concrete structures due to the loss of moisture by evaporation is known as concrete shrinkage or shrinkage of concrete. It is a time dependent deformation which reduces the volume of concrete without the impact of external forces. Types of Shrinkage in Concrete:

Tounderstandthisaspectmoreclosely, shrinkage can be classified in the following way:

- (a) PlasticShrinkageinconcrete
- (b) DryingShrinkageinconcrete
- (c) AutogeneousShrinkageinconcrete
- (d) CarbonationShrinkageinconcrete

The Types of shrinkage are explained as below:

a. PlasticShrinkage:

Plastic shrinkage is contraction in volume due towater movement from the concrete while still in theplastic state, or before it sets. This movement of water can be during the hydration process or from the environmental conditions leading to evaporation of water that resides on the surface on the wetconcrete. So, the more the concrete bleeds, the greater the plastic shrinkage should be. Plastic shrinkage is proportional to cement content and, therefore, inversely proportional to the w/c ratio.

Plastic shrinkage in concrete can be reduced mainly by preventing the rapid lossof water from surface. This can be done by covering the surface with polyethylene sheeting immediately on finishing operation; by fog spray that keeps the surface moist; or by working at night. Use of small quantity of aluminium powder is also suggested to offset the effect of plastic shrinkage. Similarly, expansive cementor shrinkage compensating cement also can be used for controlling the shrinkage during the setting of concrete.

b. DryingShrinkage:

Just as the hydration of cement is an everlasting process, the drying shrinkage is also an everlastingprocess when concrete is subjected to drying conditions. The drying shrinkage of concrete is analogousto the mechanism of drying of timber specimen. The loss of free water contained in hardened concrete, does not result in any appreciable dimension change. It is the loss of water held in gel pores that causes the change in the volume. Under drying conditions, the gel water is lost progressively over a long time, as long as the concrete is kept in drying conditions. Cement paste shrinks more than mortar and mortarshrinks more than concrete. Concrete made with smaller size aggregate shrinks more than concretemade with biggersize aggregate. The magnitude of drying shrinkage is also a function of the fineness of gel. The finer the gelthemore is the shrinkage.

c. AutogeneousShrinkage:

Autogeneous shrinkage, also known as "basic shrinkage," is the shrinkage due to chemical reactionsbetween cement with water, known as hydration, and do not include environmental effects such astemperature and moisture changes. Its magnitude is usually ignored in concretes with w/c more than 0.40.

In a conservative system i.e. where no moisture movement to or from the paste is permitted, whentemperature is constant some shrinkage may occur. The shrinkage of such a conservative system isknown as autogeneous shrinkage. Autogeneous shrinkage is of minor importance and is not applicable in practice to many situations except that of mass of concrete in the interior of a concrete dam.

d. CarbonationShrinkage:

Carbon dioxide presentin the atmosphere reactsin the presence of water withhydrated cement.Calciumhydroxide[Ca(OH)²]

getsconvertedtocalciumcarbonateandalsosomeothercementcompounds are decomposed. Such a complete decomposition of calcium compound in hydrated cementis chemically possible even at the low pressure of carbon dioxide in normal atmosphere. Carbonationpenetrates beyond the exposed surface of concrete very slowly. The rate of penetration of carbondioxide depends also on the moisture content of the concrete and the relative humidity of the ambientmedium. Carbonation is accompanied by an increase in weight of the concrete and by shrinkage.

Carbonation shrinkage is probably caused by the dissolution of crystals of calcium hydroxide anddeposition of calcium carbonate in its place. As the new product is less in volume than the productreplaced, shrinkage takes place. Carbonation of concrete also results in increased strength and reducedpermeability, possibly because water released by carbonation promotes the process of hydration and also calcium carbonate reduces the voids within the cement paste. As the magnitude of carbonationshrinkage is very small when compared to long term drying shrinkage, this aspect is not of much significance

MODULE IV
MIXDESIGN

The various factors affecting the choice of concrete mixdesignare:

1. Compressive strength of concrete

- Concretecompressivestrengthconsideredasthemostimportantconcreteproperty.Itinflue ncesmanyotherdescribablepropertiesofthehardenedconcrete.
- Themeancompressive strength (fcm) required at a specificage, usually 28 days, determines the nominal water-cementration of themix.
- ISO 456-200, British Standard, and Eurocode utilize the term mean compressivestrength which is slightly greater than characteristic compressive strength. However, ACI Codedonotuse such term.

Other factorswhichinfluences the concrete compressive strength at given time and curedataspecified temperature is compaction degree.

• Concretecompressivestrengthisinverselyproportional to the water-cementratio.

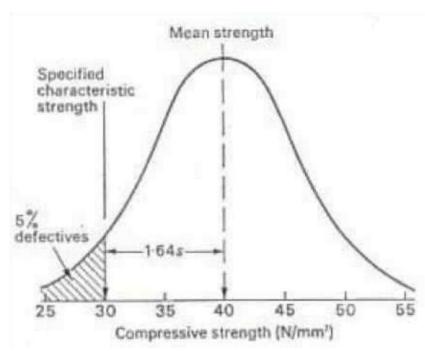


Fig.1:Means compressivestrengthvs characteristiccompressivestrength



Fig.2:compressivestrengthof concrete

2. Workabilityofconcrete

 $Concrete work ability for satisfactory placement and compaction depends on the size and {\tt concrete} work ability for satisfactory placement and {\tt concrete} work$ shapeof the section to be concreted, the amount and spacing of reinforcement, and concrete transportation; placement; and compaction technique. Additionally, use highworkability concrete for the narrow and complicated sectionwithnumerouscornersorinaccessibleparts. This will ensure the achievement of full c ompactionwithareasonableamountofeffort.

- Slumptestvaluesusedtoevaluateconcreteworkability.
- Slumptestvaluesforvariousreinforcedconcretesectionsrangesfrom25mmto175mm.

3. Durabilityofconcrete

- The ability of concrete to withstand harmful environment conditions termed asconcrete durability.
- High strengthconcreteisgenerallymoredurablethanlowstrengthconcrete.
- In the situations when the high strength is not necessary but the conditions of exposure are such that high durability is vital, the durability requirement willdetermine the utilized water-cementratio.
- Concretedurabilitydecreaseswiththeincreaseofw/cratio.

4. Maximumnominalsizeofaggregate

- Reinforcementspacingcontrolsmaximumaggregatesize.
- Aggregatesizeis
 inverselyproportionaltocementrequirementforwatercementratio. This is because work
 ability is directly proportional to size of aggregate
- However, the compressive strength tends to increase with the decrease in size of aggregate. Smaller aggregate size offers greater surface area for bonding with mortarmixthat gives higher strength.
- IS456:2000andIS1343:1980recommendsthatthenominalsizeoftheaggregateshouldbeas largeaspossible.
- Finally,inaccordancewithACIcode,maximumaggregatesizeshallnotexceedminimumrei nforcement spacing,bar diameter,or25mm.



Fig.3:Maximumaggregatesize

5. Gradingandtypeofaggregate

- Aggregategradinginfluencesthemixproportionsforaspecifiedworkabilityandwaterceme ntratio.
 - Therelative proportions between coarse and fine aggregate in concrete mixinfluence concrete strength.
- Wellgradedfineandcoarseaggregateproduceadenseconcretebecauseoftheachievement ofultimatepackingdensity.
- Ifavailableaggregate, which obtained from natural source, does not confirm to the specified grading, the proportioning of two or more aggregate become essential.
- Additionally, for specific workability and water to cement ratio, type of aggregateaffectsaggregatetocementratio.
- Lastly, Animportant feature of a satisfactory aggregate is the uniformity of the grading that achieved by mixing different size fractions.

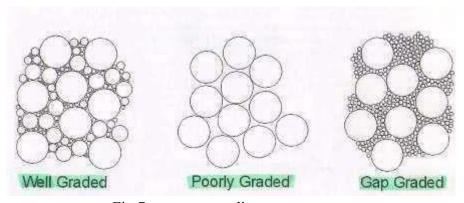


Fig.7:aggregategradingtypes

6. QualityControlatsite

- Thedegree of control could be evaluated by the variations intestresults.
- The variation in strength results from the variations in the properties of the mixing redients, in addition to lack of control of accuracy in batching, mixing, placing, curin gandtesting.
- Finally, the lower the difference between the mean and minimum strengths of themixlowerwillbethecementcontentrequired. The factor controlling this difference is term edas quality control.

ACCEPTANCECRITERIAFORDESIGN MIXCONCRETE

The concrete shall be deemed to comply with the strength requirements if:

a) Everysamplehasateststrengthnotlessthanthecharacteristic value; or

b) The strength of one or more samples though less than the characteristic value, is in each case not less than the greater of:

1) The characteristic strengthminus 1.35 times the standard deviation; and

2) 0.80timesthecharacteristicstrength ; and the average strength of all the samples is not less than the characteristic strength plus

$$\left[1.65 - \frac{1.65}{\sqrt{\text{No. of samples}}}\right]$$

timesthestandarddeviation.

II. The concrete shall be deemed not to comply with the strength requirements if:

a) The strength of any sample is less than the greater of:

1) the characteristic strength mix is 1.35 times the standard deviation; and

2) 0.80timesthecharacteristicstrength; or

b)Theaveragestrengthofallsamplesislessthanthecharacteristicstrength

$$\left[1.65 - \frac{3}{\sqrt{\text{No. of samples}}}\right]$$

pius

timesthestandarddeviation.

IIIConcretewhichdoesnotmeet

the strength requirements as specified in I, but has a strength greater than that required by II may, at the discretion of the designer, be

acceptedasbeingstructurallyadequatewithoutfurthertesting.

IV. Concreteofeachgradeshallbeassessed separately.

VConcreteshallbeassesseddailyfor compliance.

VI. Concrete is liable to be rejected if it is porous or honey-combed; its placing has been interrupted without providing a proper construction joint; therein forcement has been

$$f_{ek} + \left\{1.65 - \frac{1.65}{\eta}\right\} \times s$$

displaced beyond the tolerances specified; or construction tolerances have not been met. However, the hardened concrete may be accepted after carrying outsuitable remedial measures to the satisfaction.

VIII. Wherethevalue of the averagestrength of the tests (preferably 30 tests or 15 tests) is less than

shallberejected.

CONCRETEMIXPROPERTIONING:-

Determination of the proportion of the concrete ingredients such ascement, fine aggregate, coarse aggregate, water and admixtures is called concrete mix. • A proper mix design only can give the specified properties such as workability, strength, permeability and durability with economy.

MethodsofProportioning Concrete

- ArbitraryMethodofProportioning Concrete.
- FinenessModulusMethod ofProportioningConcrete.
- MinimumVoidMethod.
- MaximumDensityMethod:
- Water CementRatioMethodofProportioningConcrete.

ARBITRARYMETHODOFPROPORTIONINGCONCRETE:-

Thegeneral expression for the propertions of cement, sandand coarse aggregare in 1:n:2nby volume.

- 1:1:2and1:1.2::2.4forveryhighstrength
- 1:1.5:3and1:2:4fornormalworks
- 1:3:6and1:4:8forfoundationsandmassconcreteworks.

Recommendedmixesofconcrete: TheconcreteasperIS 456:2000,thegrades

ofconcretelowerthanM20arenottobeusedinRCCwork.

M10	1:3:6
M15	1:2:4
M20	1:1.5:3
M25	1:1:2

Fineness Modulus Method of Proportioning Concrete:

The term fineness modulus is used to indicate an index number which i roughlyproportionaltotheaveragesizeoftheparticleintheentirequantity of aggregates.

The fineness modulus is obtained by adding the percentage of weight of the material retained on the following sieve and divided by 100.

The coarser the aggregates, the higher the fineness modulus. Sieve is adopted for:

Allaggregates:80mm,40mm,20mm,10mm,andNos.480,240,120,60,30and15.

Coarseaggregates:mm,40mm,20mm,10mm,andNo.480.

Fineaggregates: Nos. 480, 240, 120, 60, 30 and 15.

Proportionofthefineaggregatetothecombinedaggregatebyweight

$$R = \frac{P_2 - P}{P - P_1} \times 100$$

Where, P = desired fineness modulus for a concrete mix of fine and coarse aggregates.

P₁ = finenessmodulus offineaggregate

 P_2 = finenessmodulusofcoarseaggregate.

MinimumVoidMethod:-

Itdoesnotgivesatisfactoryresult.

The quantity of sand used should be such that it completely fills the voids of coarseaggregate. Similarly, the quantity of cement used shown such that it fills thevoids of sand, so that adense mixtheminimum voids is obtained.

In actual practice, the quantity of fine aggregate used in the mix is about 10% more than thevoids in the coarse aggregate and the quantity of cement is kept as about 15% more than thevoidsinthefineaggregate.

MaximumDensityMethod:This methodisnot veryPopular.

Itisdeterminedbytheformulaas

$$P = 100 \left(\frac{d}{D}\right)^{1/2}$$

Where,D = maximumsizeofaggregate(i.e.coarseaggregate)

P = percentageofmaterialfinerthandiameterd(byweight)d = maximumsizeoffineaggregate.

Aboxisfilledwithvaryingproportionsoffineandcoarseaggregates. The proportion which gives heaviest weight is then adopted.

Water - CementRatioMethodofProportioningConcrete:-

According to the water – cement ratio lawgiven by Abram as a result of many experiments, the strength of wellcompacted concrete with good workability is dependent only on the ratio.

- The lowerwatercontentproducesstiffpaste having greaterbinding property andhence the lowering the water-cement ratio within certain limits results in theincreased strength.
- Similarly, the higherwatercontentincreasesthe workability, butlowerthe strengthof concrete.
- Theoptimumwatercementratiofortheconcreteofrequiredcompressivestrengthisdecidedfrom graphs and expressions developed from various experiments.
- Amountofwaterlessthantheoptimumwaterdecreasesthestrengthandabout10%less may be insufficient to ensure complete setting of cement. An increase of 10%above the optimum may decrease the strength approximately by 15% while anincreasein50%maydecreasethestrengthtoone-half.
- According to Abram's Lawwater-cementlaw, lesser thewater-cementratio in aworkable mixgreater will be the strength.
- Ifwatercementratioislessthan 0.4 to 0.5, completely dration will not be secured. Some practical values of water cementratio for structure reinforced concrete
- 0.45for 1:1:2concrete
- 0.5for 1:1.5:3concrete
- 0.5to0.6for 1:2:4concrete.

Concretevibratedbyefficientmechanicalvibratorsrequirelesswatercementratio, and hence have more strength.

ThumbRules for decidingthequantityofwaterinconcrete:

- (i) Weightofwater = 28%oftheweightofcement + 4%oftheweightof totalaggregate
- (ii) (ii) Weightofwater = 30% of the weight of cement + 5% of the weight of total aggregate.

BIS METHODOFMIXDESIGN:-

The process of selecting suitable ingredients of concrete and determining their relativeamountswiththeobjectiveofproducingaconcreteoftherequired, strength, durability, and workability as economically as possible, is termed the concrete mixdesign. Procedure for concrete mixdesign requires following step by step process:

- 1. Calculationoftargetstrengthofconcrete
- 2. Selection of water-cementratio
- 3. Determinationofaggregateaircontent

- 4. Selectionofwatercontent forconcrete
- 5. Selectionofcement content forconcrete
- 6. Calculationofaggregateratio
- 7. Calculation of aggregate content for concrete
- 8. Trialmixesfortestingconcretemixdesignstrength

Step1:CalculationofTargetStrengthofConcrete:-

 $Targets trength is denoted by f_twhich is \\$

obtained by characteristic compressive strength of concrete at 28 days (f_{ck}) and value of standard deviation (s)

$$f_t = f_{ck} + 1.65s$$

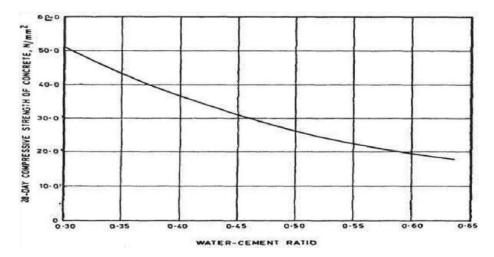
Standarddeviationcanbetakenfrombelowtable

Gradeofconcrete	Standarddeviation(N/mm²)
M10	3.5
M15	3.5
M20	4.0
M25	4.0
M30	5.0
M35	5.0
M40	5.0
M45	5.0
M50	5.0

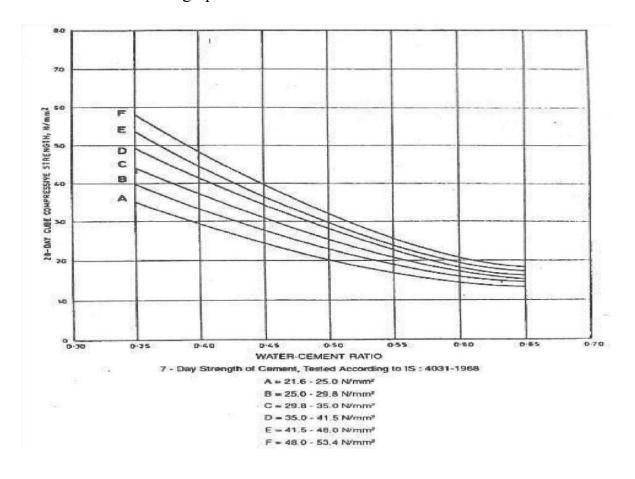
Step2:Selection of Water-CementRatio:-

Ratio of the weight of water to weight of cement in the concrete mix is water-cement ratio. It is the important

considerationinconcretemixdesigntomaketheconcreteworkable. Watercementratio is selected from the below curve for 28 days characteristic compressive strength of concrete.



Similarly, we can determine the water-cementration from the 7-day concrete strength, the curves are divided on the basis of strength from water cement ratio is decided. Which is observed from the below graph.



Step3:DeterminationofAggregateAircontent:-

Nominalmaximumsizeofaggregate	Aircontent (% ofvolume ofconcrete)

10mm	5%
20mm	2%
40mm	1%

Air content in the concrete mix is determined by the nominal maximum size of aggregateused.Belowtable willgive the entrappedair content in percentage of volume of concrete.

Step 4:SelectionofWaterContentforConcrete:-

Selectthe watercontentwhich is useful to get required workability with

thehelpofnominalmaximum size of aggregate as given in below table. The table given below is used whenonly angular shaped aggregates are used in concrete as well as the slump should be 25 to 50mm.

Nominalmaximumsizeofaggregate	Maximumwatercontent
10mm	208
20mm	186
40mm	165

If the shape of aggregate or slump value is differing from above, then some adjustments are required as follows.

MODULEV SPECIALCONCRETE

LIGHTWEIGHTCONCRETE:-

Lightweightconcretecanbedefinedasatypeofconcretewhichincludesanexpandingagentinthatitincreases the volume of the mixture while giving additional qualities such as liability and lessened thedeadweight. It is lighter than the conventional concrete

.

The material unit cost of lightweight concrete is typically higher than that of normal-weight concrete, but the unit cost usually is more than offset by the overall reduction in concrete volume and steeltonnage for the structural system.

Thebenefitsofusinglightweightaggregateconcreteinclude:

- Reductionindeadloadsmakingsavingsinfoundationsandreinforcement.
- Improved thermal properties.
- Improved fireresistance.
- Savings intransportingandhandlingprecastunitsonsite.
- Reductioninformworkandpropping.

Disadvantages:-

- Itisverysensitive withwater contentinthemixture.
- It is difficult to place and finis because of the porosity and angularity of the aggregate.
- Mixingtimeislongerthanconventionalconcretetoassurepropermixing.

Lightweightaggregateconcrete:-

Light weight aggregate is a type of coarse aggregate that is used in the production of lightweightconcreteproductssuchasconcreteblock, structural concrete,

and pavement. The expanded material has properties similar to natural aggregate, but is less dense and therefore yields a lighter concrete product.

Foam concrete, also known as Lightweight Cellular Concrete (LCC) or Low Density Cellular Concrete(LDCC) is defined as a cement-based slurry, with a minimum of 20% (per volume) foam entrained into the plastic mortar.

The concrete has a minimum 28-day compressive strength of 2500 psi (17 M Pa), an equilibriumdensitybetween70and 120lb/ft³ (1120 and1920kg/m³),and consistsentirelyoflightweightaggregateoracombinationoflightweightandnormal-densityaggregate.

<u>CELLULAR CONCRETE:</u>- Cellular concrete is a lightweight cement-based material, containingmanygasbubblesevenlydistributed in the volume, produced by blending and maturing of a mixture of cement, filler, water, agent generating cells. By the method of generating the air or gas cells there exist foam concrete and gas concrete.

cellularconcrete is used in building and construction applications such as roof decks and floor decks, and geotechnical applications such as annular space filling in slip lining and void fill abandonment. Cellular concrete can also be found in architectural and precast applications. cellular concrete is used inbuildingandconstructionapplications such as roof

decksandfloordecks,andgeotechnicalapplicationssuchasannularspacefillinginslipliningandvoidfillabando

nment. Cellularconcretecanalsobefoundinarchitecturalandprecastapplications.

MaterialUsedinCellularConcrete:-

Thematerials used in cellular concrete which reduce the density of concrete are:

1. Cement

CellularlightweightconcreteisahomogeneouscombinationofPortlandcement,cementsilica,cementp ozzolana, lime-pozzolana; lime-silica pastes having identical cell structure obtained using gasformingchemicalsoffoamingagents atmeasuredlevels. 2. FlyAsh

Asflyashisaby-productanditsdisposalisveryexpensive. It is one of the key ingredients which resolve the issue of disposal and at thesametime, it's very economical making iten vironmentally friendly.

3. Foam

ThemainconstituentoftherawmaterialoffoamthatisusedintheproductionofcellularconcreteisGenfiland its organic substance. The size of the bubbles differs from around 0.1 to 1.5 mm indiameter. Thefoamgeneratorisemployed to produce stablefoambyusinganappropriate agent.ReferenceCodesonCellular Concrete

1. ASTM C 869 – "Standard Specification for Foaming Agents Used in Making Preformed FoamforCellularConcrete"

Cellular

- 2. ASTMC796 -
 - "Standard Test Method for Foaming Agents for use in Producing Cellular Concrete using Preformed Foam"
- 3. ASTM C 495 "Standard Test Method for Compressive Strength of Lightweight Insulating

Concrete"

TypesofCellularConcretebased onDensity concreteisdifferentiatedinto3typesbasedontherangeofdensity,whichisproducedfordifferentpurpose.

1. HighDensityCellularConcrete

This is a structural grade concrete having the density ranging from 1200kg/m3 to 1800 kg/m3. It is used in the construction of load-bearing walls, partition walls and in the production of pre-cast blocksforload-bearingbrickwork.

2. MediumDensityCellularConcrete

Thedensityrangeofthiscellularconcreteis800-1000

kg/m3. Themajoruse of this type of cellular concrete is found in the manufacturing precast blocks for nonload bearing brickwork.

3. LightDensityCellularConcrete

Light density cellular concrete has a density in the range of 400 –600 kg/m3. LDCC is ideal for thermalandsoundinsulations. Theyactasaresistanceagainstfireaccidents, termiteandmoistureabsorbent. Theyhavealsoprovedtobeabettersubstitutethanglasswool, woodwooland

thermocol. Advantages of Cellular Concrete:

1. Lightweight

Thelowweightpropertyofcellular concretehasagreatadvantageonbuildingdeadloadsandcraningworks.

2. FireResistance

Theair pocketsformedactsasabarrierfor fire. The structure made of cellular concrete is non-combustible and can endure fire breakout for hours.

3. ThermalInsulation

Cellular concrete acts aperfect thermal insulator.

4. AcousticalInsulation

Thelowdensityincreasesacousticalinsulation.

5. EnvironmentalFriendly

Fly-ashbasedcellularlightweightconcreteissuitableforsurroundingbecausefly-ashisoneoftheby-productsofindustrialwaste.6.Cost-Efficient

Costofthematerialusedisconcreteisreducedasthefoamisintroduced intotheconcrete. Secondly, the use of industrial waste such as fly-ash saves a considerable amount of investment on cementproducts.

7. Other Advantages

Cellularlight-weightconcreteisalsotermite-proofandresistanttowardsfreezing issues.

NON-FINESCONCRETE:-

HIGHDENSITYCONCRETE:-

Highdensityconcreteisa concretehavingadensityin therange of 6000 to 6400 kg/cum.

High density concrete is also known as Heavy weight concrete. High density concrete is mainly used for the purpose of radiation shielding, for counterweights and other uses where high density is required.

The high density concrete has a better shielding property, so that it can protect harmful radiations likeXrays,gammarays,andneutrons.

High density aggregatesare used toachieveheavy weightconcrete. Some of the high density aggregates are barite, ferrophosphorus, limonite, hematite, limonite, magnetite, goethite, steel punching, and steel shots. The point to remember is that in order to achieve this type of concrete, high fixed water content is required.

The selection of the abovementioned aggregates mainly depend upon the physical properties likebulk density, specific gravity, availability of materials, and its expenses. In order to achieve work ability, high density the aggregates should be free from dirt, oil or grease stains and other foreignmatter. Or else, it will retard the hydration process and the effective bonding of particles. Some of the boron additions like colemanite, borocalcite are used in the preparation of concrete to improve the shielding properties. These additions may affect the setting time of concrete so, trial mixes should be made and tested depending upon the suitability.

FIBRE REINFORCEDCONCRETE:-

FibreReinforced Concrete can be defined as a composite material consisting of mixtures of cement,mortarorconcreteanddiscontinuous,discrete,uniformlydispersed suitablefibres.

Fibre is a small piece of reinforcing material possessing certain characteristics properties. They can be circular or flat. The fibre is often described by a convenient parameter called "aspect ratio". The aspect ratio of the fibre is the ratio of its length to its diameter. Typical aspect ratio ranges from 30 to 150.

Fibre-reinforcement is mainly used in shotcrete, but can also be used in normal concrete. Fibre-reinforced normal concrete are mostly used for on-ground floors and pavements, but can beconsideredforawiderangeofconstructionparts(beams, pliers, foundationsetc)eitheraloneorwithhandtiedrebars.

NECESSITYOFFIBERREINFORCEDCONCRETE:-

- Itincreasesthe tensile strengthoftheconcrete.
- Itreducestheairvoidsandwatervoidstheinherentporosityofgel.
- Itincreases the durability of the concrete.
- Fibressuch as graphite and glass have excellent resistance to creep, while the same is not trueformostresins. Therefore, the orientation and volume of fibreshave a significant influence on the creepper formance of rebars/tendons.
- Reinforced concrete itself is a composite material, where the reinforcement acts as the strengthening fibre and the concrete as the material. It is therefore imperative that the behaviour under the reinforcement are materials be similar so that the differential deformations of concrete and the reinforcement are minimized.
- It has been recognized that the addition of small, closely spaced and uniformly dispersedfibres to concrete would act as crack arrester and would substantially improve its staticanddynamic properties.

Polymerconcretes

Polymer concretes are a type of concrete that use polymers to replace lime-type cements as a binder. In some cases the polymer is used in addition to Portland cement to form Polymer Cement Concrete (PCC) or Polymer Modified Concrete (PMC).

COMPOSITION:-

In polymer concrete, thermoplastic polymers are used, but more typically thermosetting resins are used as the principal polymer component due to their high thermal stability and resistance to awide variety of chemicals. Polymer concrete is also composed of aggregates that include silica, quartz, granite, limestone, and other high quality material. The aggregate must be of good quality, free of dust and other debris, and dry. Failure to fulfil these criteria can reduce the bondstrength between the polymer binder and the aggregate. USES:-

- Polymer concrete may be used for new construction or repairing of old concrete.
 Theadhesiveproperties of polymer concrete allow repair of both polymer and conventional cementbasedconcretes.
- The corrosion resistance and low permeability of polymer concrete allows it to be used inswimming pools, sewer structure applications, drainage channels, electrolytic cells for basemetalrecovery, and other structures that contain liquids or corrosive chemicals.
- It is especially suited to the construction and rehabilitation of manholes due to their ability towithstand toxic and corrosive sewer gases and bacteria commonly found in sewer systems. Unlike traditional concrete structures, polymer concrete requires no coating or welding of PVC protected seams.

PROPERTIES:-

The exact properties depend on the mixture, polymer, aggregate used etc. etc. but generally speaking with mixtures used:

Thebinderismoreexpensivethancement

SignificantlygreatertensilestrengththanunreinforcedPortlandconcrete(sinceplastic is'stickier'thancement andhasreasonabletensilestrength)

SimilarorgreatercompressivestrengthtoPortlandconcrete

Much fastercuring

Good adhesion to most surfaces, including to reinforcements Goodlong-termdurabilitywithrespecttofreezeandthawcycles

Low permeability to waterand aggressive solutions Goodchemical resistance

Goodresistanceagainstcorrosion

Lighter weight (slightly less dense than traditional concrete, depending on the resincontent ofthemix)

Maybevibratedtofillvoidsinforms

Allowsuseofregular form-releaseagents(insomeapplications)

Dielectric

Product hard to manipulate with conventional tools such as drills and presses due to itsdensity. Recommendgetting pre-modified product from the manufacturer

Smallboxesaremorecostlywhencomparedtoitsprecastcounterparthoweverprecastconcretes induction of stacking or steel covers quickly bridge the gap.

Polymers are added to the concrete mixes either in the form of an aqueous emulsion or in a dispersed form. This is to improve the following properties of concrete:

- 1. The extensibility and the tensile strength of the concrete structure
- 2. Theimpactresistance
- 3. The Abrasion resistance
- 4. The durability and the resistance to the aggressive fluids
- 5. Thebondbetweentheoldand thenewconcrete

The polymer modified cement concrete is a composite that is obtained by the incorporation of apolymeric material into the concrete. This is carried out during the mixing stage. The polymers that are incorporated at this stage should innow a yinterfere with the hydration process.

TypesofPolymerModifiedConcrete(PMC)

Based on the type of the modifier used, polymer modified concrete can be classified into the following types:

- 1. Latex-ModifiedCementConcrete
- 2. Pre-PolymerModifiedCementConcrete

Latex-ModifiedCementConcrete

Thelattices are white milk likes uspensions that consist of very small sized polymer particles, that are suspended in water with the help of emulsifiers and stabilizing agents. It contains about 50% of polymer solid by mass.

Both elastomeric and glassy polymers have been employed in the lattices for modifying cementconcrete. These imparthighmodulus of elasticity, higherstrength and low rate of corrosion of there inforcement.

Thelatexadmixturescanbestoredinanadequateexposurethathelpsthemixturetobeprotectedagainstthefree zingandtemperatureexposures.

If the mixtures to reddrum is exposed to sunlight, it has to be properly covered with a suitable insulating blanket. The enclosed temperature to be maintained must be less than 85F.

Thenaturalrubberlatex,acrylonitrile-butadiene,neopreneandrubberlatexaresomeofthecommonly used elastomeric latex. The glass polymers have a greater modulus of elasticity, strengthandtheyundergoabrittle typeoffailure.

The common examples of the same are polyvinylidene chloride, acrylic polymers, polyvinyl acetateand styrene butadiene copolymer latex. The polyvinylidene copolymer is used in unreinforcedconcrete applications due to the presence of residual chlorine that causes the corrosion of reinforcement.

Moist curing of composite for a period of one to seven days forms the optimum curing temperature. This is later followed by a dry curing. At the 28 days of curing, the latex modified concrete wouldhave gainedits final strength.

Prepolymers-Modified Cement Concrete

The polyester -styrene based system, epoxy system and the furan system etc. are used for this. Thestrengthimprovement of this type of PMC is of the order of 50-100% over the conventional concrete.

Its adhesion property is good and has the improved durability properties. The tensile strength and themodulus of rupture are more than twice those of the conventional concrete. The formation of themicro-crackingislessinthe case of pre-polymer-modified cement concrete.

There is lower water cement ratio and the filling of pores within the pre polymer modified concretewhichwill helpinimproving the durability of the concrete compared with the conventional concrete. But the cost of the polymer is not accounted in the strength gain of the polymer modified with the conventional concrete.

odified concrete.

Properties of Polymer Modified Concrete

The properties of Polymer modified concrete are explained below:

- The addition of polymers makes the concrete mix to become more workable. This can hencereduce theamountofwaterthatisaddedtothe concretemix.
- The crushing strength of the concrete is increased using the polymerine on crete. This is because the polymer reduces the water cement ratio which in turn increase the crushing strength.
- Thebondbetweentheaggregateandthematrixisimproved.
- The polymer modification increases the flexural strength of the concrete.
- The polymer modified concrete consist of at least 3 % more amount of entrained air than theplain concrete. This additional amount of entrained air will reduce the modulus of elasticity of the concrete (PMC).
- Thepolymer additionincreasesthesettingtimeofconcrete.
- Theresistanceoftheconcreteagainstabrasion is increased using the polymer.
- Freezingandthawingresistanceoftheconcretestructureisimprovedbypolymermodification.
- Thepenetrationofchlorineionsandotherdeleteriousmaterialsisrestricted. The PMC gain higher esistance against such undesirable effects.
- The PMC gains superior shear bondstrength.
- The ductility property of the polymer modified concrete is more compared with the conventional concrete.
- Thesegainasuperiortensileandflexuralstrengthcomparedtotheconventionalconcrete.

ApplicationsofPolymerModifiedConcrete:-

- The polymer modified concrete can be used in the repair and the rehabilitation of old damaged concrete.
- Thefloorconstructioninfrozen foodfactoriesgainsgreatapplication. This is because of the higher freeze and thaw resistance of PMCs.
- For floor construction of factories were chances of the splitting of chemicals andoilsmore pronetohappen.
- Forthepreparation of steel bridge and ship decks surfaces.
- Fortheconcretestructurethatismoresubjectedto largedosesofde-icingsalts.

HIGHPERFORMANCECONCRETE:-

- Highperformanceconcreteisaconcretemixture, which possess high durability and high strength when compared to conventional concrete.
- Thisconcretecontainsoneormoreofcementiousmaterialssuchasflyash,Silicafumeorground granulatedblastfurnace slagandusuallya superplasticizer.
- Theterm'highperformance'issomewhatpretentiousbecausetheessentialfeatureofthisconcrete isthatit'singredientsand proportionsarespecificallychosensoasto

have particularly appropriate properties for the expected use of the structure such as highstrengthandlowpermeability.

- Hence High performance concrete is not a special type of concrete. It comprises of the samematerials as that of the conventional cement concrete. The use of some mineral and chemical admixtures like Silicafume and Super plasticizer enhance the strength, durability and work ability qualities to a very high extent.
- High Performance concrete works out to be economical, even though it's initial cost is higherthanthatofconventionalconcretebecausetheuseofHighPerformanceconcreteinconstruc tion enhances the service life of the structure and the structure suffers less damagewhichwouldreduceoverallcosts.
- Concreteisadurableandversatileconstructionmaterial. Itisnotonly
- Strong, economical and takes the shape of the form in which it is placed, butit is also aesthetically satisfying. However experience has shown that concrete is vulnerable to deterior ation, unless precautionary measures are taken during the design and production. For this we need to understand the influence of components on the behaviour of concrete and toproduce a concrete mix within closely controlled to learness.

The conventional Portland cement concrete is found deficient in respect of:

- Durabilityinsevereenvirons(shorterservice lifeandfrequentmaintenance)
- Timeofconstruction(slowergainofstrength)
- Energyabsorptioncapacity(forearthquakeresistantstructures)•Repairandretrofittingjobs.

Hence it has been increasingly realized that besides strength, there are other equally important criteriasuch as durability, workability and toughness. And hence we talk about 'High performance concrete'where performance requirements can be different than high strength and can vary from application toapplication.

SELFCOMPACTING CONCRETE(SCC):-

Selfcompactingconcrete(SCC)canbedefinedasfreshconcretethatflowsunderitsownweightanddoes not require external vibration to undergo compaction. It is used in the construction where it ishardtousevibratorsforconsolidation of concrete.

MaterialsUsedforSelfCompactingConcrete

Themaining redients used in design of self compacting concrete are:

1. Cement

OrdinaryPortlandcementeither43or53gradecementcanbeused.

2. Aggregates

ThesizeoftheaggregatesusedforSCCdesignis limitedto20mm. Ifthereinforcementemployedforthe structure is congested, the aggregate size used can be in the range 10 to 12mm. Well gradedaggregateseitherroundorcubicalshapeare abestchoice.

The fine aggregates used in SCC can be either natural aggregates or manufactured aggregates (M- Sand) with a uniform grade. The fine aggregates with particle size less than 0.125mm are generally employed.

3.Water

The quality of water used is same that followed for reinforced concrete and prestressed concrete construction.

4.MineralAdmixtures

Themineraladmixturesusedcanvarybasedonthemixdesignandproperties required. Mentioned below are the different mineral admixtures that can be used and their respective properties they provide.

Ground Granulated Blast Furnace Slag (GGBS): The use of GGBS helps to improve the rheological properties of the self compacting concrete.

Flyash: The fine flyash particles help to improve the filling of the internal concrete matrix with fewer pores. This improves the quality and durability of the SCC structures.

SilicaFumes: Theuseofsilicafumeshelpstoincreasethemechanical properties of the self compacting concret estructure.

StonePowder: Theuseofstonepowder

inSCCisusedto

 $improve the powder content of the mix. {\bf 5. Chemical Admixtures}$

Newgenerationsuper plasticizersarecommonlyusedinSCCmixdesign. Inordertoimprovethefreeze and thaw resistance of the concrete structure, air entraining agents are used. To control thesettingtime,retarders are employed.

Tests and Properties of Self Compacting Concrete

Therequirements of the self compacting concrete areachieved by the properties in its fresh state. The three main properties of SCC are:

FillingAbilityTests	PassingAbilityTests	SegregationResistanceTests
Slumpflowtest	L-BoxTest	V- funneltestat T5minutes
T50cm Slump Flow	J- ring test	GTM screen stability Tests
Orimet	U- BoxTest	
V-funnelTest	Fill – BoxTest	

- 1. **FillingAbility:** This property of the concrete is the ability to flow under its own weight without any vibra tion provided intentionally.
- 2. PassingAbility: This property is the ability of the concrete to maintain it shomogeneity.
- 3. Segregationresistance: This istheresistance of the concrete not to undergo segregation when it flows during the self compaction process. Different tests are conducted to determine the above mentioned properties of Self compacting concrete. The tests conducted for Self compacting concrete can be categorized into three categories:
 - 1. FillingAbilityTests
 - 2. PassingAbilityTests
 - 3. SegregationResistanceTest

AdvantagesofSelfCompactingConcrete

Themain advantages of selfcompacting concrete are:

- 1. The permeability of the concrete structure is decreased
- 2. SCCenablesfreedomindesigningconcretestructures
- 3. The SCC construction is faster
- 4. Theproblems associated with vibration is eliminated
- 5. The concrete is placed with ease, which results in large costs aving
- 6. Thequalityoftheconstructionisincrease
- 7. The durability and reliability of the concrete structure is high compared to normal concretestructures
- 8. Noisefromvibrationisreduced. This also reduce the handarm vibration syndrome issues

DisadvantagesofSelf

CompactingConcreteSCCconstructionfacethe followinglimitations:

- $1. \ \ There\ is no globally accepted test standard to undergo SCC mix design$
- 2. The cost of construction is cost lier than the conventional concrete construction 3. The use of designed mix will require mor etrial batches and lab tests
- 4. Themeasurementandmonitoringmustbemoreprecise.
- 5. The materialselectionforSCCismorestringent

ApplicationsofSelfCompactingConcrete

Themajorapplications of self compacting concrete are:

- 1. Construction of structures with complicated reinforcement
- 2. SCC isusedforrepairs, restoration and renewal construction

- $3. \ \ Highly stable\ and durable retaining walls are constructed with the\ help of SCC$
- 4. SCC isemployed in the construction of raft and pile foundations